

## Quality

## Traffic Assessment

# Drinking Water Treatment Plant Chief Justice Cushing Highway Scituate, Massachusetts 

## Prepared for:

Woodard \& Curran<br>250 Royall St<br>Canton, MA 02021

August 9, 2023


Accuracy


Integrity


# Traffic Assessment 

| To: | Ms, Rebecca Paustian |
| :--- | :--- |
|  | Woodard \& Curran |
|  | 250 Royall Street |
|  | Canton, MA 02021 |

Reg: Drinking Water Treatment Plant Chief Justice Cushing Hwy. Scituate, MA

Date: August 9, 2023
From: Kirsten Braun, P.E., Project Manager
Project \#: 22123

## INTRODUCTION

Chappell Engineering Associates, LLC (CEA) has conducted this Traffic Assessment for the proposed Stearns Meadow Drinking Water Treatment Plant to be located on Chief Justice Cushing Highway (Route 3A) south of Old Forge Road in Scituate, Massachusetts. Access to the site is proposed via a full access driveway on the north side of the site and an exit only driveway on the south side of the site onto Chief Justice Cushing Highway. The site location in relation to the surrounding roadways is shown on Figure 1.

This report has been prepared to estimate the project traffic generation, evaluate the safety of the site access points, and provide a qualitative assessment of the impacts of this traffic on the adjacent streets. As documented in this report, both proposed site driveway intersections with Chief Justice Cushing Highway meet or exceed the minimum required sight distances and safe operation can therefore be expected. It is recommended that any proposed landscaping, signs or fencing in the vicinity of the driveways be kept low (maximum 2 feet in height from street level) or set back sufficiently so as not to impede the available sight distances.

Traffic increases on Chief Justice Cushing Highway south of the site, are expected in the range of 39 to 41 additional vehicles during peak hours, or an increase of about one additional vehicle every one and a half minutes. North of the site, 26 to 28 additional vehicles are expected on Chief Justice Cushing Highway during peak hours, representing an increase of one additional vehicle every two to two and a half minutes. These increases are minimal and well within the daily fluctuation of traffic.

Chief Justice Cushing Highway (Route 3A) adjacent to the site is under MassDOT jurisdiction and therefore the project will require a Highway Access Permit from MassDOT for the construction of
the proposed driveways. The project does not exceed any transportation or land thresholds that would require the filing of an Environmental Notification Form (ENF).

Figure 1
Site Location Map


## TRAFFIC VOLUMES

Traffic volume and vehicle speed information along Chief Justice Cushing Highway near the site were obtained from an automatic traffic recorder (ATR) count conducted on January 31, 2023 and February 1, 2023. To determine if the count data needed to be adjusted to represent annual average month conditions consistent with Massachusetts Department of Transportation (MassDOT) guidelines for traffic impact assessment, historical traffic volume data were obtained from MassDOT's Weekday Seasonal Adjustment Factors for the latest year available. This document provides a monthly adjustment factor based on the roadway classification of the study roadways. Chief Justice Cushing Highway is classified as an urban principal arterial (U3). This roadway classification shows that the months of January and February are approximately six and three percent below average month conditions, respectively. To provide a conservative assessment, the counts were upwardly adjusted by six percent to represent average month conditions. The MassDOT Seasonal Adjustment Factors are provided in the Appendix.

The MassDOT Traffic and Safety Engineering 25\% Design Submission Guidelines were updated on May 31, 2022. These new directives note that traffic volume data collected after March 1, 2022 are no longer subject to any adjustments to represent pre-pandemic traffic volume conditions, except in areas where land use is predominantly office. Therefore, since the traffic volume data were collected in January and February of 2023 and land use in the area is predominantly residential, COVID adjustments do not need to be applied to the data. Table 1 summarizes the 2023 Existing traffic volumes on the study area roadways.

Table 1
Existing Traffic Volume Summary

| Location | Daily Volume ${ }^{\text {a }}$ | Peak Hour Volume ${ }^{\text {b }}$ |  | K-Factor ${ }^{\text {c }}$ | Directional Distribution ${ }^{\text {d }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Chief Justice Cushing | 9,378 | AM: | 934 | 10.0\% | 61\% NB |
| Highway Adjacent to the Site |  | PM: | 838 | 8.9\% | 61\% SB |

${ }^{a}$ In vehicles per day.
${ }^{\mathrm{b}}$ In vehicles per hour.
${ }^{\mathrm{c}}$ Percentage of daily traffic occurring during the peak hour.
${ }^{\mathrm{d}} \mathrm{NB}=$ northbound, $\mathrm{SB}=$ southbound.

## VEHICLE SPEEDS

Speed measurements were conducted along Chief Justice Cushing Highway adjacent to the site by measuring the elapsed time for vehicles traveling a short, pre-measured distance between two checkpoints. The travel time was recorded using automatic traffic recorders and the speed is derived by dividing the elapsed time into the measured distance between checkpoints. The results of the speed measurements are summarized in Table 2.

Table 2
Observed Travel Speeds ${ }^{\text {a }}$

| Location/Direction | Posted Speed Limit | Average Speed | $85^{\text {th }}$ Percentile Speed ${ }^{\text {b }}$ |
| :---: | :---: | :---: | :---: |
| Chief Justice Cushing Highway |  |  |  |
|  |  |  |  |
| Northbound | 50 | 47 | 52 |
| Southbound | 50 | 51 | 56 |

${ }^{\mathrm{a}}$ In miles per hour (mph).
${ }^{\mathrm{b}}$ Speed at, or below which 85 percent of all observed vehicles travel.

As shown, the average travel speeds along Chief Justice Cushing Highway adjacent to the site were found to be comparable to the posted speed limit of 50 miles per hour ( mph ) with 47 mph traveling northbound and 51 mph traveling southbound. The $85^{\text {th }}$ percentile speeds were found to be higher than the posted speed limit with 52 mph traveling northbound and 56 mph traveling southbound. The higher $85^{\text {th }}$ percentile travel speeds were accordingly used in the calculation of minimum sight distance requirements, as described below.

## SIGHT DISTANCE

To identify potential safety concerns associated with site access and egress, sight distances have been evaluated at the proposed driveway intersections with Chief Justice Cushing Highway to determine if the available sight distances for vehicles exiting the site meet or exceed the minimum distances required for approaching vehicles to safely stop. The available sight distances were compared with minimum requirements, as established by the American Association of State Highway and Transportation Officials (AASHTO). ${ }^{1}$ AASHTO is the national standard by which

[^0]vehicle sight distance is calculated, measured, and reported. The MassDOT and the Executive Office of Energy and Environmental Affairs (EEA) require the use of AASHTO sight distance standards when preparing traffic impact assessments and studies, as stated in their guidelines for traffic impact assessments.

Sight distance is the length of roadway ahead that is visible to the driver. Stopping Sight Distance (SSD) is the minimum distance required for a vehicle traveling at a certain speed to safely stop before reaching a stationary object in its path. The values are based on a driver perception and reaction time of 2.5 seconds and a braking distance calculated for wet, level pavements. When the roadway is either on an upgrade or downgrade, grade correction factors are applied. Stopping sight distance is measured from an eye height of 3.5 feet to an object height of 2 feet above street level, equivalent to the taillight height of a passenger car. The SSD is measured along the centerline of the traveled way of the major road.

Intersection sight distance (ISD) is provided on minor street approaches to allow the drivers of stopped vehicles a sufficient view of the major roadway to decide when to enter the major roadway. By definition, ISD is the minimum distance required for a motorist exiting a minor street to turn onto the major street, without being overtaken by an approaching vehicle reducing its speed from the design speed to 70 percent of the design speed. ISD is measured from an eye height of 3.5 feet to an object height of 3.5 feet above street level. The use of an object height equal to the driver eye height makes intersection sight distances reciprocal (i.e., if one driver can see another vehicle, then the driver of that vehicle can also see the first vehicle). When the minor street is on an upgrade that exceeds 3 percent, grade correction factors are applied.

SSD is generally more important as it represents the minimum distance required for safe stopping while ISD is based only upon acceptable speed reductions to the approaching traffic stream. However, the ISD must be equal to or greater than the minimum required SSD in order to provide safe operations at the intersection. In accordance with the AASHTO manual, "If the available sight distance for an entering or crossing vehicle is at least equal to the appropriate stopping sight distance for the major road, then drivers have sufficient sight distance to anticipate and avoid collisions. However, in some cases, this may require a major-road vehicle to stop or slow to accommodate the maneuver by a minor-road vehicle. To enhance traffic operations, intersection sight distances that exceed stopping sight distances are desirable along the major road." Accordingly, ISD should be at least equal to the distance required to allow a driver approaching the minor road to safely stop.

The available intersection sight distances at the site driveway intersections with Chief Justice Cushing Highway were measured and compared to minimum requirements as established by AASHTO. Since the requirements are based on the adjacent street speed of traffic, the observed $85^{\text {th }}$ percentile speeds on Chief Justice Cushing Highway were used. The required minimum sight distances for these speeds are compared to the available distances, as shown in Table 3.

Table 3
Sight Distance Summary

| Location/Direction | Sight Distance (feet) |  |  |
| :---: | :---: | :---: | :---: |
|  | Measured | $\begin{gathered} \text { Minimum } \\ \text { Required (SSD) } \\ \hline \end{gathered}$ | Desirable (ISD) ${ }^{\text {b }}$ |
| Chief Justice Cushing Highway at North Site Driveway: |  |  |  |
| North of intersection | 600+ | 557 | 555 |
| South of intersection | 600+ | 418 | 555 |
| Chief Justice Cushing Highway at South Site Driveway: |  |  |  |
| North of intersection | 600+ | 557 | 555 |
| South of intersection | 600+ | 418 | 555 |

${ }^{\text {a }}$ Values based on AASHTO SSD requirements for the observed $85^{\text {th }}$ percentile travel speed of 52 mph traveling northbound and 56 mph traveling southbound on Chief Justice Cushing Highway. A 5 percent grade factor (traveling uphill in the northbound direction) was applied to the calculation of minimum sight distance requirements.
${ }^{\mathrm{b}}$ Values based on AASHTO ISD requirements for posted speed limit of 50 mph on Chief Justice Cushing Highway.

As shown in the table, both the minimum required and desirable sight distances are exceeded at the proposed site driveway intersections with Chief Justice Cushing Highway. To ensure that minimum required sight distances are maintained at both site driveway intersections, it is recommended that any proposed landscaping, fencing, or signs in the vicinity of the driveways be kept low (maximum 2 feet in height from street level), or set back sufficiently (outside the sight triangles as defined by AASHTO) so as not to impede the available sight distances.

## TRIP GENERATION

The traffic to be generated by the proposed drinking water treatment plant was estimated using the Institute of Transportation Engineering (ITE) Trip Generation Manual. ${ }^{2}$ As proposed, the entire building will be 35,132 square feet in size with 22,413 square feet of water treatment plant process area, 7,142 square feet of operations space and 5,193 square feet of garage space. The garage portion of the development is expected to be ancillary to the drinking water treatment plant and therefore the total building size was assumed to be 29,939 square feet for trip generation purposes. To present a conservative assessment, the total building square footage was rounded up to 30,000 square feet. Based on a review of the ITE data, Land Use Code (LUC) 170 (Utilities) is most

[^1]appropriate as it includes trip generation data for water supply/control and sewage treatment facilities. Therefore, LUC 170 was used in estimating the traffic generation characteristics of the project, as shown in Table 4. The trip generation calculations are provided in the Appendix.

Table 4
Trip Generation Summary

| Time Period | Proposed <br> Drinking Water <br> Treatment Plant ${ }^{a}$ |
| :---: | :---: |
| Weekday Daily | 370 |
| Weekday AM Peak Hour |  |
| Enter | 60 |
| Exit | 9 |
| Total | 69 |
| Weekday PM Peak Hour |  |
| Enter | 12 |
| Exit | 53 |
| Total | 65 |

${ }^{\text {a }}$ ITE Land Use Code 170 (Utilities) based on 30,000 square feet.

As shown, the proposed drinking water treatment plant will generate 370 trips (185 in and 185 out) on a weekday daily basis, of which 69 trips ( 60 in and 9 out) would occur during the weekday AM peak hour and 65 trips ( 12 in and 53 out) would occur during the weekday PM peak hour.

## TRIP DISTRIBUTION

Since the development is expected to function as a workplace, the US Census Bureau's Journey to Work data for the residence of those working in Scituate was used to estimate the expected trip distribution of the site generated trips. Based on these data and the available travel routes, it is expected that 60 percent of the site traffic will be to and from the south on Chief Justice Cushing Highway and the remaining 40 percent would be to and from the north on Chief Justice Cushing Highway. The US Census Bureau's Journey to Work data are included in the Appendix. Since the southern driveway is an egress only, all traffic entering the site will utilize the northern driveway. Based on the site plan, circulation around the building will be in a counterclockwise direction with the rear drive behind the building operating one-way from the northern driveway to the southern
driveway. There is a parking area near the northern driveway where visitors and employees can park and then use the northern driveway to exit the site. Based on this information, traffic bound for both the north and south was split evenly between the northern and southern driveways.

## TRAFFIC INCREASES

Based on the traffic generation and distribution patterns, the largest increase in traffic is expected on Chief Justice Cushing Highway south of the site, where an additional 39 to 41 vehicles are expected during peak hours. These increases represent a 4.7 percent increase in traffic over the existing volumes during peak hours, or approximately one additional vehicle every one and a half minutes during peak hours. On Chief Justice Cushing Highway north of the site, an additional 26 to 28 vehicles are expected during peak hours. These increases represent a 3.3 percent increase in traffic over the existing volumes, or approximately one additional vehicle every two to two and a half minutes. These increases are minimal and well within the daily fluctuation of traffic. Furthermore, smaller increases in traffic are expected during all other hours of the day.

In addition, these increases are below the Transportation Impact Assessment (TIA) Guidelines established by MassDOT to determine the traffic study area for development projects. These guidelines specify that the study area for analysis of a development's traffic impacts should include any intersection where site-generated trips increase the peak hour traffic volume by a) five percent or more, or b) by more than 100 vehicles per hour. The traffic increases described above are below these thresholds and evaluation of additional intersections beyond the site driveways should not be required.

## SITE ACCESS

Access to the proposed site will be provided via two new curb cuts on Chief Justice Cushing Highway. The northern driveway will provide full access while the southern driveway will provide egress only. Both driveways should accordingly be designed to accommodate the largest vehicle expected to use the site providing corner radii that will allow tractor trailers to make turning movements without crossing the Chief Justice Cushing Highway centerline. At a minimum, the northern driveway should be 24 feet wide with 30 -foot corner radii consistent with MassDOT standards for commercial driveways. Since the southern driveway is proposed to be an exit-only driveway, it should be no more than 20 feet wide with a minimum 30 -foot radius for right turns out, depending on the use of this driveway by tractor trailer trucks. STOP (R1-1) signs and stop lines should be installed at each driveway exit. One-Way (R6-1) signs should be installed at the exit-only driveway along with Do Not Enter (R5-1) signs facing Chief Justice Cushing Highway to assure compliance with the one-way flow pattern.

## MEPA THRESHOLDS

The site abuts and proposes to access a state highway, Chief Justice Cushing Highway (Route 3A) and therefore will require a Highway Access Permit from MassDOT. Based on Transportation and Land thresholds established by the Massachusetts Environmental Policy Act (MEPA), review of the project is required if the project requires a Highway Access Permit from MassDOT and exceeds one or more of the following thresholds:

- Generation of 2,000 or more daily vehicle trips
- Creation of 300 or more new parking spaces
- Generation of 1,000 or more daily vehicle trips plus 150 or more parking spaces
- Creation of 5 or more acres of additional impervious surface
- Alteration of 25 or more acres of additional land
- Site is located within an Area of Critical Environmental Concern (ACEC)

As shown in Table 4, the project will generate 370 weekday daily vehicle trips and the site plan shows a total of 18 parking spaces are proposed on site. The transportation review thresholds are accordingly not exceeded. A total of 1.805 acres of impervious surface are proposed and 9.6 acres of land alteration. Accordingly, the land thresholds regarding impervious surface and land alteration will also not be exceeded. Finally, the site is not located within an ACEC as designated by the Secretary of Environmental Affairs. MEPA review of the project is therefore not required based on the transportation and land thresholds.

## CONCLUSIONS

- The project consists of constructing a drinking water treatment plant, Stearns Meadow, on vacant land adjacent to Chief Justice Cushing Highway (Route 3A) south of Old Forger Road in Scituate, Massachusetts.
- Access to the site is proposed via two new driveways on Chief Justice Cushing Highway. A full access driveway is proposed on the north side of the site and an egress only driveway on the south side.
- The development project is expected to generate 370 vehicle trips on a typical weekday (half entering and half exiting) of which 69 vehicle trips ( 60 entering and 9 exiting) occur during the weekday AM peak hour and 65 vehicle trips ( 12 entering and 53 exiting) occur during the weekday PM peak hour.
- Once distributed onto the adjacent streets, the largest increase in traffic is expected south of the site on Chief Justice Cushing Highway, where an additional 39 to 41 vehicles are expected during peak hours. These increases represent a 4.7 percent increase in traffic over
the existing volumes during peak hours, or approximately one additional vehicle every one and a half minutes.
- North of the site, an additional 26 to 28 vehicles are expected on Chief Justice Cushing Highway during peak hours. These increases represent a 3.3 percent increase in traffic over the existing volumes, or approximately one additional vehicle every two to two and a half minutes.
- The above increases in traffic are minimal and well within the daily fluctuation of traffic. These increases also do not exceed the MassDOT Transportation Impact Assessment (TIA) Guidelines that could require evaluation of additional intersections beyond the site driveways.
- Both driveways should be designed to accommodate the largest vehicle expected on site. Depending on the size trucks to use the facility, the northern driveway should be a minimum of 24 feet wide with 30 -foot corner radii consistent with MassDOT standards for commercial driveways. Since the southern driveway is proposed to be an exit-only driveway, it should be no more than 20 feet wide with a minimum 30 -foot radius for right turns out.
- $\quad$ STOP (R1-1) signs and stop lines should be installed at each driveway exit. One-Way (R61) signs should be installed at the exit-only driveway along with Do Not Enter (R5-1) signs facing Chief Justice Cushing Highway to assure compliance with the one-way flow pattern.
- The project proposes access to a state highway, Route 3A, and therefore will require a Highway Access Permit from MassDOT. The Massachusetts Environmental Policy Act (MEPA) transportation and land thresholds are not exceeded and therefore MEPA review of the project is not required based on these parameters.
- Both the minimum required and desirable sight distances are exceeded at the proposed site driveway intersections with Chief Justice Cushing Highway and therefore safe operation can be expected.
- It is recommended that any proposed landscaping or signs in the vicinity of the site driveways be kept low to the ground (less than two feet above street level) or set back sufficiently so as not to impede sight distances for drivers exiting the site.


## APPENDIX

Traffic Count and Vehicle Speed Data
Traffic Count Adjustment Data
Trip Generation Calculations
Journey-to-Work Data

## Accurate Counts

978-664-2565

Location : Route 3A
22123001
Location : South of Old Forge Road
City/State: Scituate, MA

| Direction: NB |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/31/2023 | 0-15 | $>15-$20 MPH | > $20-$ | $>25-$ | $>30-$ | $>35-$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| Time | MPH |  | 25 MPH | 30 MPH | 35 MPH | 40 MPH |  |  |  |  |  |  |  |  |
| 12:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 1 | 0 | 0 | 0 | 0 | 4 |
| 1:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |
| 2:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 |
| 3:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 2 | 0 | 0 | 3 |
| 4:00 | 0 | 0 | 0 | 0 | 0 | 3 | 2 | 5 | 2 | 1 | 0 | 0 | 0 | 13 |
| 5:00 | 0 | 0 | 0 | 0 | 0 | 4 | 15 | 26 | 5 | 1 | 0 | 0 | 0 | 51 |
| 6:00 | 0 | 0 | 0 | 2 | 3 | 22 | 66 | 74 | 33 | 4 | 0 | 0 | 0 | 204 |
| 7:00 | 0 | 0 | 0 | 0 | 2 | 48 | 191 | 193 | 71 | 7 | 2 | 0 | 0 | 514 |
| 8:00 | 0 | 0 | 0 | 0 | 3 | 25 | 153 | 191 | 95 | 14 | 3 | 0 | 0 | 484 |
| 9:00 | 0 | 1 | 0 | 0 | 5 | 9 | 79 | 111 | 70 | 15 | 0 | 0 | 1 | 291 |
| 10:00 | 0 | 0 | 0 | 0 | 4 | 18 | 104 | 85 | 58 | 15 | 1 | 0 | 0 | 285 |
| 11:00 | 0 | 0 | 0 | 0 | 0 | 16 | 91 | 82 | 56 | 13 | 1 | 2 | 0 | 261 |
| 12:00 PM | 0 | 0 | 0 | 5 | 12 | 22 | 84 | 85 | 45 | 7 | 0 | 0 | 0 | 260 |
| 1:00 | 0 | 0 | 0 | 0 | 5 | 20 | 71 | 93 | 67 | 7 | 1 | 0 | 0 | 264 |
| 2:00 | 0 | 0 | 0 | 0 | 0 | 12 | 111 | 104 | 74 | 14 | 1 | 0 | 0 | 316 |
| 3:00 | 0 | 0 | 0 | 0 | 3 | 16 | 71 | 127 | 100 | 20 | 2 | 0 | 0 | 339 |
| 4:00 | 0 | 0 | 0 | 0 | 0 | 11 | 69 | 128 | 72 | 18 | 2 | 1 | 1 | 302 |
| 5:00 | 0 | 0 | 0 | 0 | 5 | 27 | 86 | 111 | 42 | 10 | 1 | 1 | 1 | 284 |
| 6:00 | 0 | 0 | 0 | 0 | 2 | 17 | 73 | 77 | 38 | 7 | 0 | 0 | 0 | 214 |
| 7:00 | 0 | 0 | 0 | 0 | 0 | 15 | 36 | 55 | 19 | 2 | 2 | 0 | 0 | 129 |
| 8:00 | 0 | 0 | 0 | 0 | 0 | 5 | 37 | 36 | 17 | 3 | 0 | 0 | 0 | 98 |
| 9:00 | 0 | 0 | 0 | 1 | 0 | 10 | 18 | 30 | 13 | 1 | 2 | 0 | 0 | 75 |
| 10:00 | 0 | 0 | 0 | 0 | 1 | 2 | 7 | 7 | 3 | 0 | 0 | 0 | 0 | 20 |
| 11:00 | 0 | 0 | 0 | 0 | 0 | 2 | 4 | 4 | 7 | 0 | 0 | 0 | 0 | 17 |
| Total | 0 | 1 | 0 | 8 | 45 | 305 | 1369 | 1627 | 889 | 159 | 20 | 4 | 3 | 4430 |
| Percentile |  |  |  | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
| Speed |  |  |  | 42 | 47 | 52 | 55 |  |  |  |  |  |  |  |
| Mean Speed (Average) |  |  |  | 46.7 |  |  |  |  |  |  |  |  |  |  |
| 10 MPH Pace Speed |  |  |  | 40-49 |  |  |  |  |  |  |  |  |  |  |
| Number in Pace |  |  |  | 2970 |  |  |  |  |  |  |  |  |  |  |
| Percent in Pace |  |  |  | 67.0\% |  |  |  |  |  |  |  |  |  |  |
| Number > 50 MPH |  |  |  | 1075 |  |  |  |  |  |  |  |  |  |  |
| Percent > 50 MPH |  |  |  | 24.3\% |  |  |  |  |  |  |  |  |  |  |

## Accurate Counts

978-664-2565

Location : Route 3A
22123001
Location: South of Old Forge Road
City/State: Scituate, MA


## Accurate Counts

978-664-2565

Location : Route 3A
22123001
Location : South of Old Forge Road
City/State: Scituate, MA


## Accurate Counts

978-664-2565

Location: Route 3A
22123001
Location: South of Old Forge Road
City/State: Scituate, MA
Direction: SB


## Accurate Counts

978-664-2565

Location : Route 3A
22123001
Location : South of Old Forge Road
City/State: Scituate, MA


## Accurate Counts

978-664-2565

Location : Route 3A
22123001
Location : South of Old Forge Road
City/State: Scituate, MA
Direction: Combined

| $\begin{array}{r} \hline \text { 2/1/2023 } \\ \text { Time } \end{array}$ | $\begin{aligned} & 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 3 | 3 | 1 | 3 | 1 | 0 | 14 |
| 1:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 4 |
| 2:00 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 4 |
| 3:00 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 2 |
| 4:00 | 0 | 0 | 0 | 0 | 1 | 2 | 7 | 5 | 4 | 2 | 0 | 0 | 0 | 21 |
| 5:00 | 0 | 0 | 0 | 0 | 0 | 17 | 16 | 23 | 12 | 9 | 2 | 0 | 0 | 79 |
| 6:00 | 0 | 0 | 1 | 5 | 18 | 55 | 101 | 72 | 31 | 4 | 0 | 0 | 0 | 287 |
| 7:00 | 0 | 1 | 1 | 13 | 12 | 28 | 177 | 266 | 163 | 45 | 11 | 2 | 1 | 720 |
| 8:00 | 0 | 0 | 0 | 1 | 3 | 50 | 186 | 289 | 247 | 90 | 13 | 2 | 0 | 881 |
| 9:00 | 0 | 1 | 0 | 0 | 5 | 24 | 90 | 211 | 184 | 61 | 18 | 1 | 1 | 596 |
| 10:00 | 0 | 0 | 0 | 0 | 5 | 24 | 96 | 168 | 158 | 43 | 17 | 1 | 0 | 512 |
| 11:00 | 0 | 0 | 0 | 0 | 10 | 33 | 119 | 177 | 157 | 38 | 12 | 0 | 0 | 546 |
| 12:00 PM | 1 | 0 | 0 | 0 | 2 | 14 | 98 | 167 | 156 | 71 | 14 | 1 | 0 | 524 |
| 1:00 | 0 | 0 | 0 | 0 | 0 | 17 | 93 | 202 | 172 | 46 | 11 | 2 | 0 | 543 |
| 2:00 | 0 | 0 | 0 | 1 | 6 | 17 | 86 | 226 | 202 | 66 | 9 | 1 | 0 | 614 |
| 3:00 | 0 | 0 | 0 | 0 | 2 | 26 | 185 | 294 | 268 | 56 | 13 | 3 | 0 | 847 |
| 4:00 | 0 | 0 | 0 | 0 | 4 | 21 | 115 | 319 | 257 | 59 | 12 | 4 | 0 | 791 |
| 5:00 | 0 | 0 | 0 | 2 | 14 | 90 | 252 | 239 | 74 | 20 | 2 | 0 | 0 | 693 |
| 6:00 | 0 | 1 | 0 | 0 | 5 | 40 | 111 | 136 | 98 | 34 | 1 | 2 | 0 | 428 |
| 7:00 | 0 | 1 | 0 | 0 | 4 | 21 | 76 | 111 | 67 | 21 | 10 | 0 | 0 | 311 |
| 8:00 | 0 | 0 | 1 | 0 | 1 | 12 | 48 | 76 | 57 | 18 | 1 | 0 | 0 | 214 |
| 9:00 | 0 | 0 | 0 | 0 | 0 | 5 | 27 | 43 | 29 | 9 | 0 | 0 | 0 | 113 |
| 10:00 | 0 | 0 | 0 | 0 | 0 | 3 | 18 | 18 | 17 | 3 | 8 | 0 | 0 | 67 |
| 11:00 | 0 | 0 | 0 | 0 | 1 | 0 | 3 | 2 | 1 | 0 | 0 | 0 | 0 | 7 |
| Total | 1 | 4 | 3 | 22 | 94 | 500 | 1907 | 3050 | 2358 | 697 | 157 | 22 | 3 | 8818 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 43 | 49 | 54 | 58 |  |  |  |  |  |  |  |
|  |  | n Speed | Average) | 48.1 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 45-54 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | in Pace | 5399 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 61.2\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number $>$ | 50 MPH | 3237 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent > | 50 MPH | 36.7\% |  |  |  |  |  |  |  |  |  |  |
| Grand Total | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 43 | 49 | 54 | 58 |  |  |  |  |  |  |  |
|  |  | n Speed | Average) | 48.4 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 45-54 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | r in Pace | 10892 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 61.6\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number $>$ | 50 MPH | 6682 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent $>$ | 50 MPH | 37.8\% |  |  |  |  |  |  |  |  |  |  |

## Accurate Counts

978-664-2565

Location : Route 3A
22123001
Location : South of Old Forge Road

| 1/31/2023 | NB |  | Hour Totals |  | SB |  | Hour Totals |  | Combined Totals |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Morning | Afternoon | Morning | Afternon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon |
| 12:00 | 2 | 72 |  |  | 3 | 59 |  |  |  |  |
| 12:15 | 0 | 63 |  |  | 2 | 83 |  |  |  |  |
| 12:30 | 0 | 58 |  |  | 0 | 81 |  |  |  |  |
| 12:45 | 2 | 67 | 4 | 260 | 1 | 66 | 6 | 289 | 10 | 549 |
| 1:00 | 0 | 65 |  |  | 0 | 66 |  |  |  |  |
| 1:15 | 0 | 70 |  |  | 2 | 62 |  |  |  |  |
| 1:30 | 1 | 67 |  |  | 0 | 82 |  |  |  |  |
| 1:45 | 0 | 62 | 1 | 264 | 1 | 76 | 3 | 286 | 4 | 550 |
| 2:00 | 0 | 77 |  |  | 1 | 82 |  |  |  |  |
| 2:15 | 0 | 68 |  |  | 0 | 74 |  |  |  |  |
| 2:30 | 1 | 78 |  |  | 1 | 97 |  |  |  |  |
| 2:45 | 0 | 93 | 1 | 316 | 0 | 138 | 2 | 391 | 3 | 707 |
| 3:00 | 0 | 80 |  |  | 0 | 104 |  |  |  |  |
| 3:15 | 0 | 97 |  |  | 0 | 133 |  |  |  |  |
| 3:30 | 1 | 91 |  |  | 1 | 101 |  |  |  |  |
| 3:45 | 2 | 71 | 3 | 339 | 0 | 127 | 1 | 465 | 4 | 804 |
| 4:00 | 4 | 57 |  |  | 1 | 132 |  |  |  |  |
| 4:15 | 1 | 81 |  |  | 4 | 103 |  |  |  |  |
| 4:30 | 2 | 78 |  |  | 3 | 117 |  |  |  |  |
| 4:45 | 6 | 86 | 13 | 302 | 8 | 82 | 16 | 434 | 29 | 736 |
| 5:00 | 7 | 71 |  |  | 4 | 90 |  |  |  |  |
| 5:15 | 9 | 88 |  |  | 8 | 97 |  |  |  |  |
| 5:30 | 15 | 54 |  |  | 14 | 102 |  |  |  |  |
| 5:45 | 20 | 71 | 51 | 284 | 7 | 87 | 33 | 376 | 84 | 660 |
| 6:00 | 35 | 88 |  |  | 13 | 80 |  |  |  |  |
| 6:15 | 39 | 46 |  |  | 21 | 76 |  |  |  |  |
| 6:30 | 56 | 34 |  |  | 34 | 48 |  |  |  |  |
| 6:45 | 74 | 46 | 204 | 214 | 47 | 45 | 115 | 249 | 319 | 463 |
| 7:00 | 89 | 31 |  |  | 37 | 38 |  |  |  |  |
| 7:15 | 128 | 33 |  |  | 47 | 42 |  |  |  |  |
| 7:30 | 138 | 38 |  |  | 78 | 44 |  |  |  |  |
| 7:45 | 159 | 27 | 514 | 129 | 87 | 35 | 249 | 159 | 763 | 288 |
| 8:00 | 134 | 22 |  |  | 96 | 59 |  |  |  |  |
| 8:15 | 137 | 33 |  |  | 99 | 25 |  |  |  |  |
| 8:30 | 116 | 26 |  |  | 92 | 30 |  |  |  |  |
| 8:45 | 97 | 17 | 484 | 98 | 65 | 14 | 352 | 128 | 836 | 226 |
| 9:00 | 86 | 16 |  |  | 82 | 12 |  |  |  |  |
| 9:15 | 71 | 20 |  |  | 77 | 12 |  |  |  |  |
| 9:30 | 74 | 22 |  |  | 71 | 15 |  |  |  |  |
| 9:45 | 60 | 17 | 291 | 75 | 43 | 12 | 273 | 51 | 564 | 126 |
| 10:00 | 68 | 9 |  |  | 61 | 12 |  |  |  |  |
| 10:15 | 76 | 4 |  |  | 49 | 6 |  |  |  |  |
| 10:30 | 74 | 3 |  |  | 79 | 6 |  |  |  |  |
| 10:45 | 67 | 4 | 285 | 20 | 52 | 4 | 241 | 28 | 526 | 48 |
| 11:00 | 58 | 4 |  |  | 68 | 2 |  |  |  |  |
| 11:15 | 75 | 6 |  |  | 66 | 2 |  |  |  |  |
| 11:30 | 61 | 2 |  |  | 63 | 1 |  |  |  |  |
| 11:45 | 67 | 5 | 261 | 17 | 71 | 2 | 268 | 7 | 529 | 24 |
| Total | 2112 | 2318 |  |  | 1559 | 2863 |  |  | 3671 | 5181 |
| Percent | 47.7\% | 52.3\% |  |  | 35.3\% | 64.7\% |  |  | 41.5\% | 58.5\% |

Location: Route 3A
Location: South of Old Forge Road

| City/State: Scituate, MA |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/1/2023 | NB |  | Hour Totals |  | SB |  | Hour Totals |  | Combined Totals |  |
| Time | Morning | Afternoon | Morning | Afternon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon |
| 12:00 | 1 | 68 |  |  | 2 | 75 |  |  |  |  |
| 12:15 | 1 | 77 |  |  | 2 | - 65 |  |  |  |  |
| 12:30 | 3 | 49 |  |  | 1 | 77 |  |  |  |  |
| 12:45 | 1 | 50 | 6 | 244 | 3 | 63 | 8 | 280 | 14 | 524 |
| 1:00 | 1 | 58 |  |  | 1 | 72 |  |  |  |  |
| 1:15 | 0 | 61 |  |  | 1 | 65 |  |  |  |  |
| 1:30 | 0 | 79 |  |  | 0 | 65 |  |  |  |  |
| 1:45 | 0 | 64 | 1 | 262 | 1 | 79 | 3 | 281 | 4 | 543 |
| 2:00 | 0 | 62 |  |  | 0 | 72 |  |  |  |  |
| 2:15 | 0 | 78 |  |  | 0 | 55 |  |  |  |  |
| 2:30 | 1 | 65 |  |  | 1 | 108 |  |  |  |  |
| 2:45 | 0 | 67 | 1 | 272 | 2 | 107 | 3 | 342 | 4 | 614 |
| 3:00 | 0 | 78 |  |  | 0 | 120 |  |  |  |  |
| 3:15 | 0 | 90 |  |  | 0 | 121 |  |  |  |  |
| 3:30 | 1 | 103 |  |  | 0 | 102 |  |  |  |  |
| 3:45 | 1 | 100 | 2 | 371 | 0 | 133 | 0 | 476 | 2 | 847 |
| 4:00 | 2 | 75 |  |  | 0 | 138 |  |  |  |  |
| 4:15 | 1 | 76 |  |  | 3 | 133 |  |  |  |  |
| 4:30 | 1 | 79 |  |  | 2 | 119 |  |  |  |  |
| 4:45 | 5 | 76 | 9 | 306 | 7 | 95 | 12 | 485 | 21 | 791 |
| 5:00 | 4 | 74 |  |  | 11 | 110 |  |  |  |  |
| 5:15 | 11 | 83 |  |  | 11 | 109 |  |  |  |  |
| 5:30 | 11 | 68 |  |  | 5 | 92 |  |  |  |  |
| 5:45 | 19 | 74 | 45 | 299 | 7 | 83 | 34 | 394 | 79 | 693 |
| 6:00 | 23 | 55 |  |  | 11 | 83 |  |  |  |  |
| 6:15 | 42 | 40 |  |  | 23 | 59 |  |  |  |  |
| 6:30 | 57 | 40 |  |  | 35 | 60 |  |  |  |  |
| 6:45 | 64 | 40 | 186 | 175 | 32 | 51 | 101 | 253 | 287 | 428 |
| 7:00 | 96 | 43 |  |  | 37 | 53 |  |  |  |  |
| 7:15 | 118 | 37 |  |  | 46 | 42 |  |  |  |  |
| 7:30 | 128 | 31 |  |  | 66 | 39 |  |  |  |  |
| 7:45 | 144 | 33 | 486 | 144 | 85 | 33 | 234 | 167 | 720 | 311 |
| 8:00 | 112 | 32 |  |  | 106 | 39 |  |  |  |  |
| 8:15 | 152 | 36 |  |  | 76 | 29 |  |  |  |  |
| 8:30 | 145 | 24 |  |  | 67 | 16 |  |  |  |  |
| 8:45 | 124 | 19 | 533 | 111 | 99 | 19 | 348 | 103 | 881 | 214 |
| 9:00 | 117 | 25 |  |  | 73 | 23 |  |  |  |  |
| 9:15 | 54 | 13 |  |  | 85 | 10 |  |  |  |  |
| 9:30 | 73 | 11 |  |  | 62 | 8 |  |  |  |  |
| 9:45 | 68 | 6 | 312 | 55 | 64 | 17 | 284 | 58 | 596 | 113 |
| 10:00 | 84 | 8 |  |  | 58 | 12 |  |  |  |  |
| 10:15 | 66 | 17 |  |  | 60 | 9 |  |  |  |  |
| 10:30 | 72 | 8 |  |  | 70 | 5 |  |  |  |  |
| 10:45 | 49 | 3 | 271 | 36 | 53 | 5 | 241 | 31 | 512 | 67 |
| 11:00 | 68 | 6 |  |  | 70 | 1 |  |  |  |  |
| 11:15 | 78 | 2 |  |  | 59 | 2 |  |  |  |  |
| 11:30 | 85 | 1 |  |  | 59 | 1 |  |  |  |  |
| 11:45 | 62 | 3 | 293 | 12 | 65 | 8 | 253 | 12 | 546 | 24 |
| Total | 2145 | 2287 |  |  | 1521 | 2882 |  |  | 3666 | 5169 |
| Percent | 48.4\% | 51.6\% |  |  | 34.5\% | 65.5\% |  |  | 41.5\% | 58.5\% |
| Grand Total | 4257 | 4605 |  |  | 3080 | 5745 |  |  | 7337 | 10350 |
| Percent | 48.0\% | 52.0\% |  |  | 34.9\% | 65.1\% |  |  | 41.5\% | 58.5\% |

Location: South of Old Forge Road

Massachusetts Highway Department Statewide Traffic Data Collection 2019 Weekday Seasonal Factors

| Factor Group | JAN | FEB | MAR | APR | MAY | JUN | JUL | AUG | SEP | OCT | NOV | DEC | Axle Factor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| R1 | 1.22 | 1.14 | 1.12 | 1.06 | 1.00 | 0.96 | 0.87 | 0.85 | 0.96 | 0.99 | 1.04 | 1.12 | 0.85 |
| R2 | 0.95 | 0.96 | 0.98 | 0.97 | 0.97 | 0.93 | 0.97 | 0.94 | 0.96 | 0.90 | 0.92 | 0.93 | 0.96 |
| R3 | 1.15 | 1.06 | 1.07 | 1.00 | 0.89 | 0.88 | 0.89 | 0.89 | 0.95 | 0.92 | 1.02 | 1.01 | 0.97 |
| R4-R7 | 1.09 | 1.09 | 1.11 | 1.02 | 0.96 | 0.92 | 0.89 | 0.89 | 0.99 | 0.98 | 1.09 | 1.13 | 0.98 |
| U1-Boston | 1.03 | 1.01 | 0.98 | 0.94 | 0.94 | 0.92 | 0.95 | 0.93 | 0.94 | 0.94 | 0.97 | 1.04 | 0.96 |
| U1-Essex | 1.09 | 1.06 | 1.03 | 0.99 | 0.94 | 0.90 | 0.88 | 0.86 | 0.93 | 0.94 | 0.99 | 1.06 | 0.93 |
| U1-Southeast | 1.06 | 1.05 | 1.01 | 0.97 | 0.95 | 0.93 | 0.93 | 0.90 | 0.94 | 0.94 | 0.98 | 1.04 | 0.98 |
| U1-West | 1.19 | 1.14 | 1.09 | 0.95 | 0.92 | 0.89 | 0.89 | 0.86 | 0.91 | 0.95 | 0.97 | 1.07 | 0.84 |
| U1-Worcester | 1.02 | 1.04 | 0.97 | 0.94 | 0.93 | 0.91 | 0.95 | 0.91 | 0.93 | 0.92 | 0.95 | 1.10 | 0.88 |
| U2 | 1.01 | 1.00 | 0.94 | 0.93 | 0.91 | 0.89 | 0.93 | 0.90 | 0.90 | 0.91 | 0.94 | 1.02 | 0.99 |
| U3 | 1.06 | 1.03 | 0.98 | 0.94 | 0.93 | 0.91 | 0.95 | 0.91 | 0.92 | 0.93 | 0.97 | 1.00 | 0.98 |
| U4-U7 | 1.01 | 1.00 | 0.95 | 0.92 | 0.88 | 0.86 | 0.92 | 0.91 | 0.92 | 0.94 | 0.99 | 1.04 | 0.99 |
| Rec - East | 1.04 | 1.16 | 1.12 | 0.98 | 0.92 | 0.88 | 0.77 | 0.81 | 0.94 | 1.02 | 1.08 | 1.12 | 0.99 |
| Rec - West | 1.30 | 1.23 | 1.32 | 1.18 | 0.95 | 0.82 | 0.70 | 0.69 | 0.97 | 0.96 | 1.16 | 1.15 | 0.98 |

urban minor artierla rural major collector

[^2] $7014,7079,7080,7090,7091,7092,7093,7094,7095,7096,7097,7108$ and 7178), Martha's Vineyard and Nantucket. Recreational - West Group - Continuous Stations 2 and 189 including stations
1 - Interstate
2 - Freeway and Expressway 3 - Other Principal Arterial 4 - Minor Arterial 5 - Major Collector
7 - Local Road and Street $4,1116,2196,2197$ and 2198.

# Institute of Transportation Engineers (ITE); 11th Edition 

 Land Use Code (LUC) 170 - Utilities| Average Vehicle Trips Ends vs: | 1000 Sq. Feet Gross Floor Area |
| :--- | :--- | :--- |
| Independent Variable (X): $\quad 30.000$ |  |


| Average Weekday daily |  |
| :---: | :---: |
| $\operatorname{Ln~T~}=0.74 \operatorname{Ln}(\mathrm{X})+2.73$ |  |
| $\mathrm{Ln} \mathrm{T}=5.25$ |  |
| $\mathrm{T}=189.97$ |  |
| $\mathrm{T}=190 \quad$ vehicle trips |  |
| with 50\% ( 95 vpd ) entering and 50\% ( | $95 \mathrm{vpd})$ exiting. |

Weekday Evening Peak Hour Of Adjacent Street Traffic
$\operatorname{Ln} T=0.81 \operatorname{Ln}(X)+0.86$
$\operatorname{Ln} \mathrm{T}=3.61$
$\mathrm{T}=37.15$
$\mathrm{T}=37$
vehicle trips
with $18 \%$ ( 7 vpd ) entering and $82 \%$ ( 30 vpd) exiting.

WEEKDAY DAILY Average Rate
$\mathrm{T}=12.29 *(\mathrm{X})$
$\mathrm{T}=368.70$
$\mathrm{T}=369 \quad$ vehicle trips
with $50 \%$ ( $185 \quad \mathrm{vpd})$ entering and $50 \%$ ( 184 vpd ) exiting.

Weekday Morning Peak Hour Of Adjacent Street Traffic

$$
\mathrm{T}=2.33 *(\mathrm{X})
$$

$$
\mathrm{T}=69.90
$$

$$
\mathrm{T}=69
$$

vehicle trips

$$
\text { with } 87 \% \text { ( } \quad 60 \quad \mathrm{vpd} \text { ) entering and } 13 \%\left(\begin{array}{lll}
9 & \mathrm{vpd}) \text { exiting. }
\end{array}\right.
$$

Weekday Morning Peak Hour Of Adjacent Street Traffic $\mathrm{T}=2.16$ * $(\mathrm{X})$
$\mathrm{T}=64.80$
$\mathrm{T}=65 \quad$ vehicle trips
with $18 \%$ ( $12 \quad \mathrm{vpd})$ entering and $82 \%$ ( 53 vpd$)$ exiting.

| 둔운 |  |
| :---: | :---: |
| 反్రీ |  |
|  |  |


| $\left\|\begin{array}{c} \overline{0} \\ \stackrel{0}{0} \end{array}\right\|$ |  |  |
| :---: | :---: | :---: |
| $\left\|\begin{array}{l} \stackrel{\varsigma}{m} \\ \check{0} \end{array}\right\|$ |  |  |
| $\left\|\begin{array}{l} \overleftarrow{~} \\ \tilde{o} \\ \check{c} \end{array}\right\|$ |  |  |


| Residance Place of Work |  | \# of Trips | \% of Trips |
| :--- | :--- | ---: | ---: |
| Scituate town | Scituate town | 1,923 | $53.0 \%$ |
| Marshfield town | Scituate town | 298 | $8.2 \%$ |
| Weymouth Town city | Scituate town | 174 | $4.8 \%$ |
| Brockton city | Scituate town | 166 | $4.6 \%$ |
| Hanover town | Scituate town | 120 | $3.3 \%$ |
| Quincy city | Scituate town | 98 | $2.7 \%$ |
| Hingham town | Scituate town | 90 | $2.5 \%$ |
| Duxbury town | Scituate town | 89 | $2.5 \%$ |
| Plymouth town | Scituate town | 88 | $2.4 \%$ |
| East Bridgewater town | Scituate town | 77 |  |
|  |  |  | $2.1 \%$ |
| Yarmouth town | Scituate town | 74 | $2.0 \%$ |
| Norwell town | Scituate town | 74 | $2.0 \%$ |
| Hull town | Scituate town | 73 | $2.0 \%$ |
| Abington town | Scituate town | 64 | $1.8 \%$ |
| Kingston town | Scituate town | 58 | $1.6 \%$ |
| Whitman town | Scituate town | 58 | $1.6 \%$ |
| Braintree Town city | Scituate town | 54 | $1.5 \%$ |
| Cohasset town | Scituate town | 52 | $1.4 \%$ |
|  |  | 3,630 |  |


[^0]:    ${ }^{1}$ A Policy on Geometric Design of Highways and Streets, $7^{\text {th }}$ Edition; American Association of State Highway and Transportation Officials (AASHTO); 2018.

[^1]:    ${ }^{2}$ Trip Generation Manual, $11^{\text {th }}$ Edition; Institute of Transportation Engineers; Washington, DC; 2021.

[^2]:    Recreational - East Group - Cape Cod (all towns) including the town of Plymouth south of Route 3A (stations

